



## SECTION SF 30 BLOCK 14 CONTINUATION PAGE

**SUMMARY OF CHANGES**

**A. BID OPENING DATE:** The bid opening date is NOT EXTENDED by this Amendment; therefore, the bids are due by 2:00 p.m. CDT on March 24, 2009. Detailed information regarding the submission of bids is located in Block No. 13 on Page 1 of the Solicitation document (SF 1442).

**B. AMENDED PROJECT SPECIFICATION SECTIONS:** The following Specification Section has been amended as delineated below; therefore bidders shall remove the affected page and replace it with the amended page attached hereto.

- Section 35 49 13.10 03 METAL SHEET PILING, page 7, paragraph 3.5 has been changed. The amended section contains page 7 only.

**C. AMENDED PROJECT DRAWING SHEETS:** One drawing sheet has been amended as delineated below; therefore bidders shall replace the affected sheet of the IFB with the amended sheet attached hereto.

- Drawing sheet G-03

**D. USACE ANSWER'S TO CONTRACTOR'S QUESTIONS:** Provided below are the USACE-Chicago District's answers to questions that were submitted by Contractor's over the past week prior to the effective date of this Amendment.

**Question 1:** Are we to include excavation and shaping of the existing edge of stream in the price of Riprap, or will this be included in the spoil bank removal. A Typical cross section is shown on sheet C-30, however there are multiple places where the existing stream bank slopes down, then benches across, then proceeds downward to the streambed elevation. Assuming that you do not want riprap to cover these benched areas, the bench should be removed prior to the installation of the Riprap.

**Answer 1:** Excavation and shaping of the existing edge of stream should be included in the price of Riprap, see paragraphs 1.2.5.1 and 1.2.6.1 of Specification Section 01 22 00.00 10, Measurement and Payment. Benches on the existing stream bank should only be removed if it prevents the Contractor from placing the riprap according to the Riprap Placement Detail shown on drawing sheet C-30.

**Question 2:** Concerning the Precast alternative – On sheet S-24, section B there is a 6" x 6" x ½" plate with a shear stud welded to it which is embedded into the precast panels, which will all be made uniformly. The plate will be welded the clip angles, which are also welded to the flange of the sheet piling. If the piling interlocks are not perfectly spaced, or if they are spaced slightly greater or smaller than nominal dimension, these clip angles will not match up properly with both the flange of the piling and the 6x6 plate. How should this be accounted for in the design of the precast panels?

**Answer 2:** The 6x6 inch embedded plates were sized to allow for play in the interlocks when the piling is driven. While the clip angles may not line up exactly on the center of the plates as shown in Section B/S24-S24 and the other details on Drawing Sheet S-24 the COE feels that the error over the four pile sections that each pre-cast panel will span will be minimal and within the assumed design tolerances.

**Question 3:** In examining Addendum 2 issued on 16 March, 2009 it appears that some quantities have changed, specifically Bid Item 0012AA, but no new bid sheets were included with the Addendum. Will new bid sheets be issued for this project?

**Answer 3:** There is no change to the bid quantity for Bid Item 0012AA.

**Question 4:** I am writing to verify the requirements for precasters to be considered for the Little Calumet Flood Protection project (February 17, 2009 solicitation). In the concrete section of the specifications for this project, it is stated that a precaster must be a "PCI certified precaster".

Our company, S & S Precast, Inc., is not certified by PCI, but we are an NPCA (National Precast Concrete Association) certified plant. We have been a precast manufacturer since 1992, and have been producing wall panels for mechanically stabilized earth retaining walls since 1999. We have produced these panels in accordance with the specifications and special provisions of the departments of transportation in the states of Indiana, Illinois, Michigan and Ohio, as well as several toll highway authorities.

The certification process for the NPCA is very thorough, covering manufacturing and product quality, materials testing, record keeping, plant safety, and many other issues. This certification should qualify our company to produce the precast panels for this project.

**Answer 4:** NPCA plant certification is acceptable; see specification section 03 30 00.00, paragraph 2.1.1, Precast Manufacturer's Qualifications revised by way of amendment #2.

**Question 5:** Typical Pipe bedding detail shows impervious fill, water stop material, and flowable fill. This detail is typically used for sewers crossing through the flood protection system. Storm sewers not crossing the flood protection system that are handling simple local surface drainage (like the 18" on this project) would not require these water-proofing materials. Typically, those "regular" sewers would just be bedded in sand or stone, then backfilled with native material (or granular backfill if under pavement). So the question is: Does this detail only apply to the sewers crossing the flood protection system? If so, can the 18" landward storm sewer that does not cross the flood protection system be backfilled in native material?

**Answer 5:** The Typical Pipe Bedding Detail applies to all the sewer pipes shown in the plans.

**Question 6:** Final Grade at wall is shown to be 598.75 on the typical sections. The detail cross sections (Sheets C15 – C26) show to "excavate existing spoil berm to El. 598.75..." However, there is 6" of topsoil to go on top of any disturbed area. Should the top of the topsoil be at 598.75 or do we excavate to 598.75 and then put on 6" topsoil? If the top of the topsoil is to be at 598.75, how is that "additional" 6" of excavation paid? Will it be paid under the excavation item or is it incidental to the topsoil?

**Answer 6:** The topsoil should be at elevation 598.75. The additional excavation necessary for the 6" of topsoil has been accounted for and should be included under bid item 0018AA, Spoil Bank Excavation.

**E. POINT OF CONTACT:** The point of contact for this Amendment is Regina G. Blair at (312) 846-5371.

(End of Summary of Changes)

### 3.3.2 Pulling and Redriving

The Contractor shall pull selected pilings after driving to determine the condition of the underground portions of pilings when directed by the Contracting Officer. The method of pulling pilings must be approved by the Contracting Officer. Any piling so pulled and found to be damaged to the extent that its usefulness in the structure is impaired shall be removed and replaced at the Contractor's expense. Pilings pulled and found to be in satisfactory condition shall be redriven when directed by the Contracting Officer.

### 3.4 REMOVAL

The removal of pilings shall consist of pulling, sorting, cleaning, inventorying and storing previously installed pilings as shown on the drawings and directed by the Contracting Officer.

#### 3.4.1 Pulling

The method of pulling piling must be approved by the Contracting Officer. Pulling holes shall be provided in pilings as required. Extractors shall be of suitable type and size. Care shall be exercised during the pulling of pilings to avoid damaging piling interlocks and adjacent construction. If the Contracting Officer determine that adjacent permanent construction has been damaged during pulling the Contractor will be required to repair this construction at no cost to the Government. Pilings shall be pulled one sheet at a time. Pilings fused together shall be separated prior to pulling unless the Contractor demonstrates to the satisfaction of the Contracting Officer that the pilings cannot be separated. The Contractor will not be paid for the removal of pilings damaged beyond structural use due to proper care not being exercised during pulling.

#### 3.4.2 Sorting, Cleaning, Inventorying and Storing

Pulled pilings shall be sorted, cleaned, inventoried and stored by type into groups as (1) piling usable without reconditioning, (2) piling requiring reconditioning and (3) piling damaged beyond structural use.

### 3.5 QUANTITIES

The estimated quantities of steel sheet piling listed in the bidding schedule of the contract are furnished for bidding purposes only. ~~Sheet pile quantities for payment shall consist of the square feet of piling acceptably installed.~~ Installed quantities shall consist of all piling including fabricated sections driven between the required top and bottom elevations of pilings plus any additions thereto resulting from changes in design or alignment.

### 3.6 QUALITY CONTROL

The Contractor shall establish and maintain a quality control system for the work under this section, in accordance with SECTION 01 45 04.00 03, "CONTRACTOR QUALITY CONTROL".

-- End of Section --

## STANDARD SYMBOLS

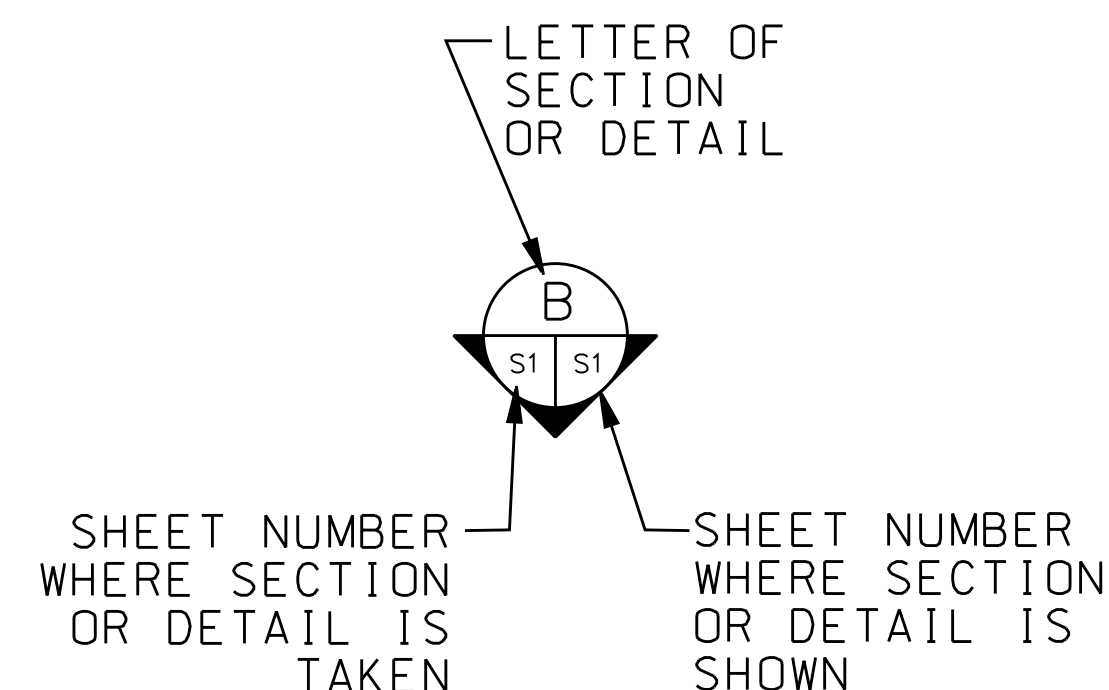


GENERAL NOTES:

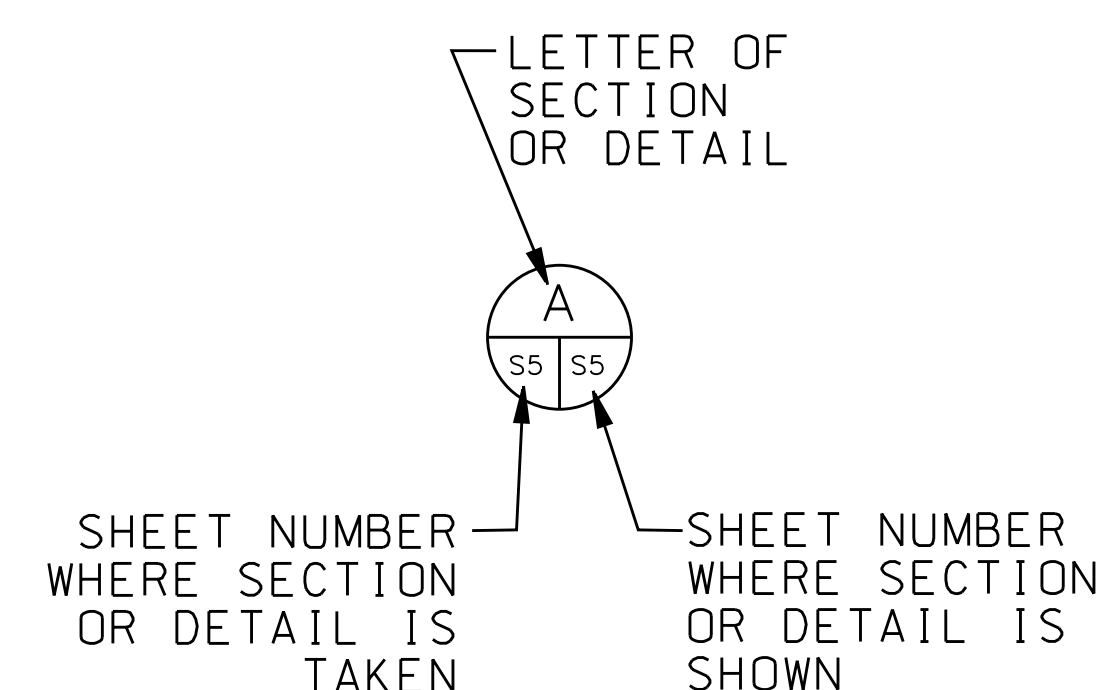
1. FOR INFORMATION ON THE FLOODWALL CONSTRUCTION SEQUENCE SEE SPECIFICATION SECTION 01 32 01.02 10.
2. THE CONTRACTOR MUST PROTECT ALL UTILITIES DURING CONSTRUCTION.
3. THE CONTRACTOR SHALL FIELD VERIFY THE EXACT LOCATIONS AND DEPTHS OF EXISTING UNDERGROUND UTILITIES.
4. UNLESS OTHERWISE NOTED, THE CONTRACTOR SHALL CONTACT UTILITY OWNERS A MINIMUM OF 30 DAYS PRIOR TO BEGINNING WORK IN THESE AREAS.
5. INDOT TEMPORARY EROSION AND SEDIMENT CONTROL GUIDELINES SHALL BE FOLLOWED THROUGHOUT THE WORK LIMITS.
6. THE GRID COORDINATE SYSTEM SHOWN ON THE PLANS IS BASED ON INDIANA STATE PLANE ZONE WEST COORDINATE SYSTEM, NORTH AMERICAN DATUM (NAD) OF 1927.
7. ELEVATIONS SHOWN REFER TO THE ELEVATION IN FEET, ABOVE THE NATIONAL GEODETIC VERTICAL DATUM (NGVD) OF 1929.
8. EXISTING FEATURES AND TOPOGRAPHY WERE REPRODUCED FROM AERIAL PHOTOGRAPHY TAKEN IN 1984 AND A LAND SURVEY PERFORMED IN 2008.
9. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF EM 385-1-1, US ARMY CORPS OF ENGINEERS, SAFETY AND HEALTH REQUIREMENTS MANUAL.
10. ALL STAGING AND STORAGE AREAS SHALL BE RESTORED TO PRE-CONSTRUCTION GRADES AND ELEVATION UNLESS DIRECTED OTHERWISE BY THE COR.
11. THE CONTRACTOR SHALL MAINTAIN THE SAME LEVEL OF FLOOD PROTECTION AS THE EXISTING LEVEE DURING CONSTRUCTION BY TEMPORARY LEVEE OR OTHER MEANS APPROVED BY THE COR. RIVER STAGES ON THE LITTLE CALUMET RIVER CAN RISE QUICKLY, SO A GAP SHOULD NEVER BE LEFT IN THE LEVEE WITHOUT EITHER THE CONTRACTOR ON SITE READY TO FLOOD FIGHT THE GAP OR PROVIDE A TEMPORARY LINE OF FLOOD PROTECTION EQUIVALENT TO THE EXISTING LINE OF FLOOD PROTECTION.
12. THE CONTRACTOR SHALL NOT PLACE ANY TEMPORARY STOCKPILES OF EARTHEN MATERIALS ON THE RIVER SIDE OF THE PROPOSED CENTERLINE OF FLOOD PROTECTION ANY TIME DURING CONSTRUCTION.
13. THE TOWN OF MUNSTER HAS PERFORMED CLEARING OF THE EXISTING TREES AND RAISED LOW AREAS OF THE EXISTING BERM TO EL. 602.0 FROM COLUMBIA AVE. TO NORTHCOTE AVE ON THE SOUTH SIDE OF THE RIVER. THIS WORK IS NOT REFLECTED IN THE PLANS OR IN THE BID SCHEDULE QUANTITIES.

## STANDARD ABBREVIATIONS

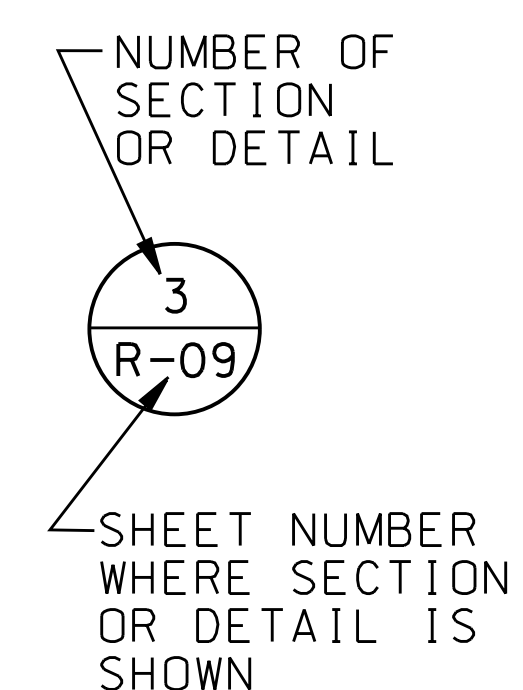
℄	CENTER LINE	U.S.	UPSTREAM
RCP	REINFORCED CONCRETE PIPE	D.S.	DOWNSTREAM
EL.	ELEVATION	CMP	CORRUGATED METAL (CULVERT) PIPE
B.M.	BENCH MARK	NGVD	NATIONAL GEODETIC VERTICAL DATUM OF 1929
T.B.M.	TEMPORARY BENCH MARK	FT	FEET
P.I.	POINT OF INTERSECTION	INV.	INVERT
P.C.	POINT OF CURVATURE	T & B	TOP & BOTTOM
P.O.B.	POINT OF BEGINNING	EF	EACH FACE
P.O.E.	POINT OF ENDING	EA	EACH
PT	POINT OF TANGENCY	SYM ABT	SYMMETRICAL ABOUT
PVI	POINT OF VERTICAL INTERSECTION	AS	ASPHALT CONCRETE
CJ	CONTRACTION JOINT	PCC	PORTLAND CEMENT CONCRETE
VERT OR V	VERTICAL	LF	LINEAR FEET
HOR OR H	HORIZONTAL	N.T.S.	NOT TO SCALE
R	RADIUS	GALV.	GALVANIZED
TYP.	TYPICAL	CNST.	CONSTRUCTION
MIN.	MINIMUM	ℙ	PLATE
MAX.	MAXIMUM	CLR.	CLEAR
DIA.	DIAMETER	PSI	POUNDS PER SQUARE INCH
O.C.	ON CENTER	EP	EDGE OF PAVEMENT
STD.	STANDARD	DIP	DUCTILE IRON PIPE
WT.	WEIGHT	WL	WATER LEVEL
REQ'D.	REQUIRED	CIP	CAST IRON PIPE
EXP.	EXPANSION	CTJ	CONTROL JOINT
EST.	ESTIMATE	EJ	EXPANSION JOINT
ⓔ	EXPANSION JOINT	EX	EXISTING
IN.	INCH	CSJ	CONSTRUCTION JOINT
∅	DIAMETER	B.O.C.	BOTTOM OF CONCRETE
EST.	ESTIMATE	S.S.	STAINLESS STEEL
T.O.P.	TOP OF PILE		



"S" SHEET SECTION MARKER

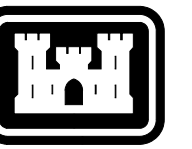


" S " SHEET SECTION TITLE



"R" SHEET SECTION TITLE

POINTS OF CONTACT			
NAME	FIRM NAME	OFFICE NAME	PHONE NUMBER
PHIL DITTMANN	BP PIPELINE	ENGINEERING	219-736-5866 EXT. 330
MARCIE FOSTER	BP PIPELINE	ROW SPECIALIST	630-836-3461
JAMES POKRAJAC	LITTLE CALUMET RIVER BASIN COMISSION	LCRBDC	219-763-0696
MARK PASYK	NIPSCO	UTILITY HIGHWAY AFFAIRS	219-647-4299
NEAL ARNDT	NIPSCO	GAS TRANSMISSION ENGINEER	219-647-4779
FRANK JANOSI	NIPSCO	DISTRIBUTION ENGINEER	219-886-5560
STAN DOSTATNI	CITY OF HAMMOND	CITY ENGINEER	219-853-6336
RICK SUTTON	CITY OF HAMMOND	HAMMOND SANITARY DISTRICT	219-853-6412 EXT. 530
JIM MANDON	TOWN OF MUNSTER	TOWN ENGINEER	219-836-6995
SHELDON EDD	US ARMY CORPS OF ENGINEERS	PROJECT ENGINEER	219-923-1763
IMAD SAMARA	US ARMY CORPS OF ENGINEERS	PROJECT MANAGER	312-846-5560



US Army Corps  
of Engineers  
Chicago District

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U.S. ARMY ENGINEER DISTRICT CORPS OF ENGINEERS CHICAGO, ILLINOIS	SIGNED BY: _____ W9126-09-B-0002 DATE: FEBRUARY 2009 SCALE: AS SHOWN DRAWN BY: JW/DB CHECKED BY: SOLIDATION NUMBER: JAG. W9126-09-B-0002 JOHN A. GROBOSKI, P.E. 7/31/08 SUBMITTED BY: _____ DATE: _____
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LITTLE CALUMET RIVER, INDIANA  
LOCAL FLOOD PROTECTION  
STAGE VII - FLOODWALL

SHEET  
REFERENCE  
NUMBER:

G-03